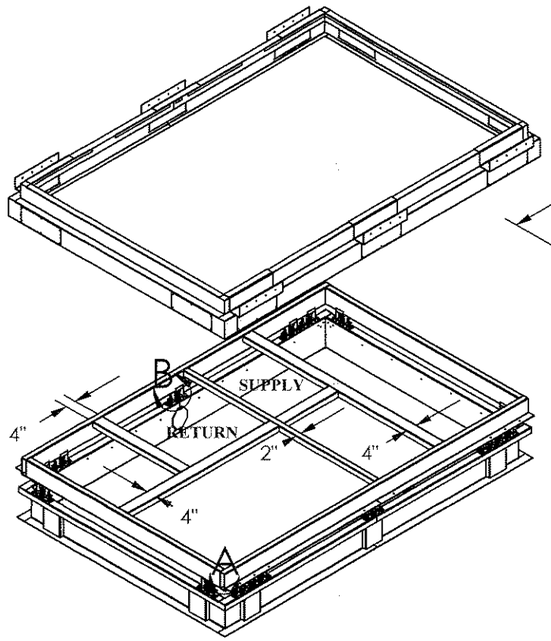
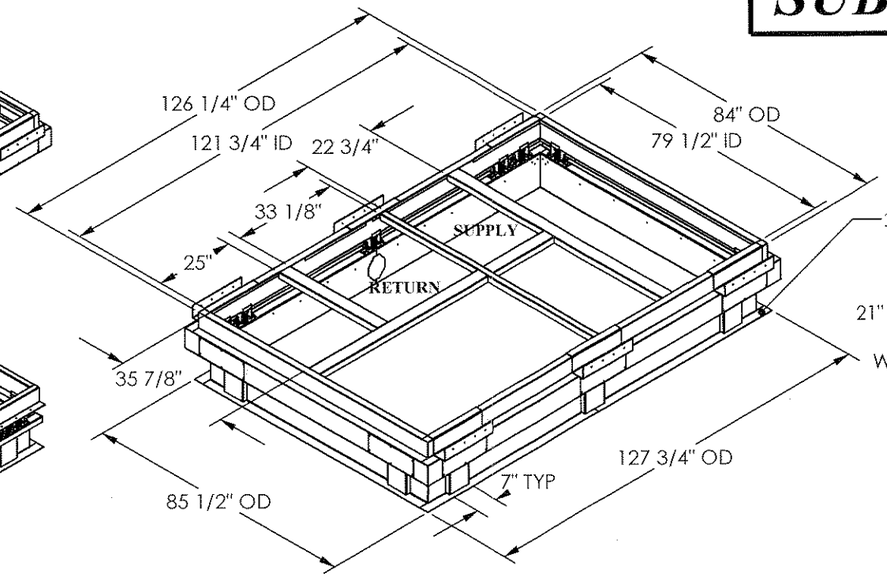
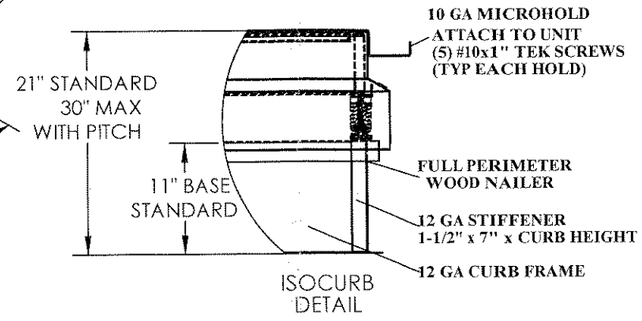
SUBMITTAL | 0403-652A SERIES

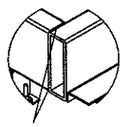




* WELDED CONSTRUCTION
* PERIMETER WOOD NAILER
* GASKET PACKAGE
* FACTORY INSTALLED HOLDDOWNS
* OSHPD PRE-APPROVED 2" DEF SEISMIC RESTRAINTS (OPA#0070)

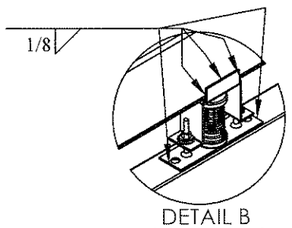


ISOCURB DETAIL



SEAM WELD TOP AND SIDES (TYP. 4 CORNERS)

DETAIL A
SCALE 1 : 10



DETAIL B
SCALE 1 : 8

2X6 TUBE STEEL SUPPORT ASSY DETAILS

FOR BOLTED ON ISOLATORS

FOR BOLTED ON ISOLATORS ADD "BOLT" TO PART#
EX: 0403-652A-01CBC-BOLT

IF BOLTED RESTRAINTS ARE PREFERRED, IT MUST BE NOTED AT TIME OF ORDER.

BOLTS WILL BE 1/2" X 2", 2 PER ISOLATOR
1/2" FLAT WASHER, 2 PER BOLT,
1/2" NUT
1/4" THICK NEOPRENE PAD IS PLACED BETWEEN RESTRAINT AND CURB
NEOPRENE CUP WILL BE UNDER SPRING.

DETAIL C
SCALE 1 : 8

REGISTERED PROFESSIONAL ENGINEER
CLIFFORD H. JOHNSON
9/2002
EXP. 6-30-07
STRUCTURAL
STATE OF CALIFORNIA
2/22/07

MicroMetl Corporation

Sparks, NV. (800) 884-4662
Indianapolis, IN. (800) 662-4822
Longview, TX. (903) 248-4800

PRODUCT NUMBER: **0403-652A ISOLATION CURBS**

0403-652A-8B-01CBC	18" TALL
0403-652A-01CBC	21" TALL
0403-652A-14B-01CBC	14" TALL

STRUCTURALLY CALCULATED VIBRATION ISOLATION CURB FOR YORK DCE, DCG, DM 240-300, DJ, DR 180-300 BCH, DEG,DEE 240

DATE: 07/2007
DRAWN BY: MAC
WEIGHT: 1040/1100/1160
MEETS SEISMIC REQUIREMENTS FOR FOLLOWING CODES:
2001 CBC
2006 IBC

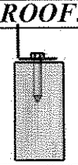
ANCHORAGE DETAILS TO ROOF:

STEEL ATTACHMENT:

SEE STEEL ATTACHMENT DETAIL SHEETS.

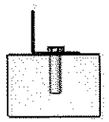
WOOD ATTACHMENT:
(DOUGLAS FIR)

(76) 1/4 x 3" SIMPSON SDS W/WASHER CENTER ON CURB FLANGE, EVENLY SPACED, (20) EACH LONG SIDE, (18) EACH SHORT SIDE



CONCRETE ATTACHMENT:
(3000 PSI MINIMUM, 4" MIN THICKNESS)
(6" MIN EDGE DISTANCE)

(26) 1/2"SIMPSON TITEN HD EVENLY SPACED,CENTER ON CURB FLANGE
8" MIN SPACING
(7) EACH LONG SIDE, (6) EACH SHORT SIDE



ENG007

BJG ARCHITECTURE + ENGINEERING

Structural Calculations
BJG# 20070133

Project:

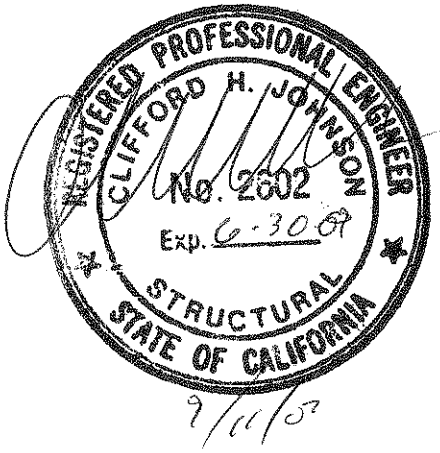
0403-652A

Prepared for:

MicroMetl Corporation
905 Southern Way
Sparks, NV 89431

Date:

August 2007



9/11/07

6995 Sierra Center Parkway Suite 200 Reno Nevada 89511 Phone 775.827.1010 Fax 775.827.1663 www.bjginc.com

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Frame and Support Curb Information **Product Number 0403-652A-01CBC**

h_{FRAME} =	30	in - Overall height from support substrate to top of curb
$h_{SUPPORT}$ =	6	in - Height of support curb from top of isolators to bottom of unit
L_{CURB} =	121.75	in - Longitudinal distance from center-to-center of transverse curb members
W_{CURB} =	79.5	in - Transverse distance from center-to-center of longitudinal curb members
h_i =	4.5	in - Height of isolator
d_i =	7.5	in - Dist. off long member end to isolator
d_{HD} =	7.5	in - Dist. off short member end to holddown

Unit Information **DH300N32**

W_P =	3085	lbs - Max. unit weight
h_{UNIT} =	52.625	in - Overall unit height above curb
h_{CM} =	35.1	in - Height above curb to center of mass
L_{UNIT} =	136.25	in - Overall unit length (longitudinal direction)
W_{UNIT} =	92	in - Overall unit length (transverse direction)

Seismic Loading - 2006 International Building Code (2006 IBC)

$Fp_{MAX} = 1.6 * S_{DS} * Ip * Wp$

Ss =	2	(2 is worst case in NV, OR, WA, AZ)
Fa =	1	(1.0 at worst case Site D, Ss ≥ 1.25)
Sms =	2	= FaSs
S_{DS} =	1.33	= 2/3 Sms
Ip =	1.5	(1.5 at worst case Occupancy)
Fp_{MAX} =	3.20	Wp
Fp_{MAX} =	2.29	Wp (ASD)
Fp_{MAX} =	7051	lb (ASD) - ASD values will be used throughout unless noted otherwise

Seismic Loading - 2001 California Building Code (2001 CBC)

$Fp_{MAX} = 4 * Ca * Ip * Wp$

Ca =	0.44	(.44 at worst case at Zone 4, Soil Type Sd)
Na =	1.5	(1.5 at worst case Seismic Source Type A <= 2km)
Ip =	1.5	(1.5 at worst case Occupancy)
Fp_{MAX} =	3.96	Wp
Fp_{MAX} =	2.83	Wp (ASD)
Fp_{MAX} =	8726	lb (ASD) - ASD values will be used throughout unless noted otherwise

Controlling Seismic Loads

Fp_{MAX} =	2.83	Wp (ASD)
Fp_{MAX} =	8726	lb (ASD) - ASD values will be used throughout unless noted otherwise

Wind Loading Check

Max. Projected Area (A_{MAX}) = h_{UNIT} * MAX (L_{UNIT} or W_{UNIT})

A_{MAX} =	7170	in^2
=	49.8	ft^2

Equivalent wind pressure required to equal seisimic loading (P_{EQ}) = Fp_{MAX} / A_{MAX}

P_{EQ} = 175 psf (ASD) **OKAY BY INSPECTION: P > 60 PSF**

Connectors from Unit to Support:

Use Self-drilling, Self Tapping Steel Screws, allowable load per Table IV-7A of the cold formed steel manual
#10 screw allowable load in 16 gage minimum material is [463] lbs each

Transverse or Longitudinal Loading

$V_{each\ side}$ = 2/3 * Fp_{MAX} (ASD)

V_{HD} = [5817] lb per side (where applicable)

Transverse Loading

Holddowns:

N_{HD} = [3] Number of holddowns per long side

R_{HD1} = (Fp_{MAX} * h_{CM}) /(N_{HD}* W_{CURB}) - 1/3 * W_P

R_{HD1} = [255] lb per HD uplift

V_{HD} = [0] lb per HD

Max Resultant Force = [255] lb per HD
Min Screws Required = [2] per HD

Isolators:

R_{MAX} = (Fp_{MAX} * (h_{cm} + h_s)) / W_{CURB} + 2/3 * W_P

R_{MAX} = [6566] lb per side - Downward

$RISO_{MIN}$ = (Fp_{MAX} * (h_{cm} + h_s)) / W_{CURB} - 1/3 * W_P

$R_{ISO\ MIN}$ = [3481] lb per side uplift

V_{ISO} =F_{pMAX}/(# Iso)

V_{ISO} = [0] lb per side

Longitudinal Loading

Holddowns:

R_{HD1} = (F_{pmax} * h_{cm}) / (2*(L_{UNIT} - d_{HD}) - 1/6*W_P

R_{HD1} = [675] lb per HD Assume all uplift into end holddowns

V_{HD} = [1939] lb per HD

Max Resultant Force = [2053] lb per HD
Min Screws Required = [5] per HD

Isolators:

R_{MAX} = (Fp_{MAX} * (h_{cm} + h_s)) / (L_{CURB} - 2d_i) + 2/3 * W_P

R_{MAX} = [5415] lb per side - Downward

$R_{ISO\ MIN}$ = (Fp_{MAX} * (h_{cm} + h_s)) / (L_{CURB} - 2d_i) - 1/3 * W_P

$R_{ISO\ MIN}$ = [2330] lb per side uplift

V_{ISO} = $V_{each\ side}$

V_{ISO} = [5817] lb per side

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Isolator Load Summary

USE 5 TYPE **OPA0070** Isolator restraints each long side for shear and vertical
USE 2 TYPE **OPA0070** Isolator restraints each short side for shear

Max. $V_{ISO} \leftrightarrow$ = V_{ISO} max. due to transverse or longitudinal loading

Max. $V_{ISO} \leftrightarrow$ = **5817** lb per side Max. $V_{ISO} \leftrightarrow$ = **831** lb each isolator

Max. $R_{ISO} \downarrow$ = max. downward force due to transverse or longitudinal loading

Max. $R_{ISO} \downarrow$ = **6566** lb per side Max. $R_{ISO} \downarrow$ = **1313** lb each isolator

Max. $R_{ISO} \uparrow$ = max. uplift force due to transverse or longitudinal loading

Max. $R_{ISO} \uparrow$ = **3481** lb per side Max. $R_{ISO} \uparrow$ = **696** lb each isolator

PRE-APPROVED MAXIMUM ALLOWABLE LOADS

Allowable Horizontal = **1000** lb each isolator **OKAY**
Allowable Vertical = **1600** lb each isolator **OKAY**

Tube Steel Support Assembly

Use HSS 2x6 Tube Steel, 6" tall, 2" wide; Use properties for hollow rectangle

Conditions and formulas per AISI Cold-Formed Steel Specification (2001) Analyze as a beam

Bending: (Per C3.1)

t =	0.125	in
Fy =	46	ksi
b =	2	in
d =	6	in
C_b =	1.14	AISC 13th ed. Table 3-1
E =	29000	ksi
G =	11500	ksi
I_y =	1.43	in^4
J =	4.50	in^4
S_x =	2.357	in^3
Ax =	1.50	in^2
$b_1 = b - 2 * t$ =	1.75	in
$d_1 = d - 2 * t$ =	5.75	in
$L = (L_{CURB} - 2 * d_i)/2$ =	53.375	in
$L_u = L / 2$ =	26.69	in
$b_{eff} = b - 3 * t$ =	1.625	in
$h_{eff} = d - 3 * t$ =	5.625	in

Allowed Lateral Unbraced Length, L_A

$L_A = 0.36 * C_b \pi / (F y S_y) * (E G J I y)^{1/2}$

L_A = **551.3** in (Eq. C3.1.2.2-1)

Ω_b = **1.67**

If laterally unbraced length is less than or equal to L_u, then the nominal moment M_n shall be used

Lu < La OKAY

$M_n = S_e F_y$

M_n / Ω_b = **64.9** k-in Table 3-12 AISC

Max moment due to center holddown, M_u

$M_u = WL^2/8$

M_u = **43808.12** lb-in

M_u = **43.81** k-in

BENDING OKAY

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Shear: (Per C3.2.1)

Ω_v =	1.60	
h / t =	48.0	
k_v =	5.34	
$\sqrt{(Ek_v / F_y)}$ =	58.0	
Aw =	1.50	in²
F_v =	27.60	ksi

F_v per Eqs. C3.2.1-2, 3, 4

Nominal Shear Strength

$V_n = A_w F_v$

V_n = 41.4 kips (Eq. C3.2.1-1)

V_n / Ω_v = 25.9 kips

Max Shear Force

$V_u = R_{MAX} / 2$

V_u = 3.28 kips **OKAY**

Web Crippling: (Per C3.4.1)

C =	7.5	
C_h =	0.048	
C_N =	0.12	
C_R =	0.08	
Ω_w =	1.75	
N =	4	in.
R =	0.25	in.
θ =	90	°

Note: N = Bearing length per isolator

Nominal Web Crippling Strength

$P_n = Ct^2F_y \sin\theta(1-C_R(R/t)^{1/2})(1+C_N(N/t)^{1/2})(1-C_h(h/t)^{1/2})$

P_n = 5.36 kips / web (Eq. C3.4.1-1)

P_n = 10.71 kips

P_n / Ω_w = 6.122 kips

$P_u = R_{MAX}$ / # of isolators per side

P_u = 1.313 kips (long side)

P_u = 2.71 kips (short side)

OKAY

Frame Assembly Stiffeners

Use 12 gage stiffener material

Conditions and formulas per AISI Cold-Formed Steel Specification (2001)

t =	0.105	in
Fy =	33	ksi
Length =	7	in
Width =	1.5	in
Height =	20	in
Ω_C =	1.8	
A =	1.03	in²
r_1 =	0.65	in
r_2 =	2.51	in
kl/r_{min} =	30.8	

$F_e = \pi^2E/(KL/r)^2$

F_e = 301.18 ksi (Eq. C4.1-1)

$\lambda_c = \sqrt{(F_y/F_e)}$

λ_c = 0.33 (Eq. C4-4)

F_n = 25.01 ksi (Eq. C4-2,3)

$P_n = A_eF_n$

P_n = 25.71 kips (Eq. C4-1)

P_n/Ω_c = 14.28 kips

$P_U = R_{MAX} / 2$

P_U = 3283.04 lbs

P_U = 3.28 kips **STIFFENER OKAY**

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Anchorage to Supporting Structure

Shear to each long side = **5817** lbs
Shear to each short side = **5817** lbs

$R_{ISO\ MIN} = (Fp_{MAX} * (h_{cm} + h_{frame})) / W_{CURB} - 1/3 * W_P$
Uplift to each long side = **6115** lbs
$R_{ISO\ MIN} = (Fp_{MAX} * (h_{cm} + h_{frame})) / (L_{CURB} - 2 * d_i) - 1/3 * W_P$
Uplift to each short side = **4292** lbs

Anchorage to Concrete Pad

4 in. thick conrete pad - min. embedment of 3 in., min. spacing of 8 in. and min. edge distance of 6 in.

w/ 1/2" Simpson Titen HD, allow = **1605** lbs in **shear**
w/ 1/2" Simpson Titen HD, allow = **1155** lbs in **tension**

Try **7** Titen HD's per long side at a minimum
Try **6** Titen HD's per short side

(Actual Shear / Allowable Shear)^(5/3) + (Actual Tension / Allowable Tension)^(5/3) ≤1.0

Elliptical Interaction Equation = **0.962** at the long sides **OK, less than 1.0**
Elliptical Interaction Equation = **0.882** at the short sides **OK, less than 1.0**

Anchorage to Wood sub-Structure

With Simpson 1/4 x 3" SDS screws...

Allow Shear = **470** lb per simpson catalog
Allow Tension = **550** lb assuming 2" penetration per NDS Table 11.2B (#14 wood screw)

7 screws required for uplift long side
5 screws required for uplift short side

13 screws required for shear both sides

20 **total screws required long side** **6.40** inches maximum spacing
18 **total screws required short side** **4.6** inches maximum spacing

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Note: Connection evaluated without consideration of bolt hole deformation.

Anchorage to Steel

With A307 1/2" Bolts...

t =	0.060	in
F_y =	33	ksi
F_u =	45	ksi
e=	1	in.
d=	1/2	in.
width=	3	in.

$R_{ISO\ MIN} = (Fp_{MAX} * (h_{cm} + h_{frame})) / W_{CURB} - 1/3 * W_P$

Uplift to each long side = **6115** lbs

$R_{ISO\ MIN} = (Fp_{MAX} * (h_{cm} + h_{frame})) / (L_{CURB} - 2 * d_i) - 1/3 * W_P$

Uplift to each short side = **4292** lbs

Shear to each long side = **5817** lbs

Shear to each short side = **5817** lbs

Design strength based on spacing and edge distance:

P_n=	2.7	kips/bolt
F_u/F_y=	1.36	
Ω=	2.00	
Φ=	0.70	
P_n/Ω=	1.35	kips/bolt
ΦP_n=	1.89	kips/bolt
3d=	1 1/2	NOTE: Distance between bolt hole centers must be greater than 3d.
1.5d=	3/4	NOTE: Distance from edge of connection to bolt hole center must be greater than 1.5d

Design strength based on bearing:

NOTE: bolt hole deformation is not considered

C=	3	in^2
m_f=	0.75	Table E3.3.1-2
Ω=	2.50	
Φ=	0.60	
P_n=	3.0375	kips/bolt
P_n/Ω=	1.215	kips/bolt
ΦP_n=	1.82	kips/bolt

Design strength based on bolt shear:

P_n=	5.3	kips/bolt	(Table IV-6)
Ω=	2.40		
Φ=	0.65		
P_n/Ω=	2.21	kips/bolt	
ΦP_n=	3.45	kips/bolt	

Governing limit state:

			Governing Limit State
P_n/Ω=	1.22	kips/bolt	Bearing Strength
ΦP_n=	1.82	kips/bolt	Bearing Strength

6	# of bolts for the long side
4	# of bolts for the short side